Wall Bracing

This document was prepared for the MBOIA and based on the wind speeds and Seismic Design Categories for Maine and shall only be used as a reference since the Wind Speeds and the Seismic Design Category for Connecticut are different.

The purpose of this document is to give you background information on wall bracing, how to calculate it and what should be shown on the plans.

If you are interested in obtaining an Excel spreadsheet to aid in calculating the required wall bracing, email me and I will forward a copy to you. Keep in mind that the spreadsheet has to be used in conjunction with the 2009 International Residential Code.



2009 IRC Wall Bracing

Maine Building Officials and Inspectors Association

MBOIA

Mark Halverson Manager – Field Services Division APA – The Engineered Wood Assn.

Bracing Topics



www.iccsafe.org Item no. 7102S09



Bracing Topics





Wind Speed







Earthquake



SDC – United States



SDC – **Northeast USA**









Effects of Forces

Racking Base Shear Overturning

Resisted by Bracing

Resisted by Anchors

Resisted by hold-downs & Dead Load







Bracing Topics





Introduction: Load Path

Vertical (Gravity) Load Path





Introduction: Load Path

Lateral (Sideways) Load Path





Introduction: Load Path



Foundation

R301.1 Application

The construction of buildings... shall result in a... complete <u>load path</u>... for the transfer of all loads... to the <u>foundation</u>.



Bracing Topics





BWP (Prescriptive)

Limitations

- 3-Stories Maximum
- Wind ≤110 mph⁽¹⁾
- SDC A-D₂
- Others (see IRC Chap. 3)
- Typically without hold-downs

Shear Walls (Engineered)

Applications

- Any building size/shape
- Wind no limit
- SDC no limit
- Calculations required

 Typically with hold-downs

(1) Wind \leq 100 mph in hurricane-prone regions.





R602.10 Wall Bracing

"Where a building, or portion thereof, does not comply with one or more of the bracing requirements in this section, those portions shall be designed and constructed in accordance with Section R301.1."











Wall Framing









Bracing Topics





History of Wall Bracing

Uniform Building Code – 1927

 All exterior walls and partitions shall be thoroughly and effectively angle braced.

Uniform Building Code – 1952

 All exterior walls and partitions shall be thoroughly and effectively angle braced <u>or sheathed with approved panels</u> adequately nailed along all edges.



History of Wall Bracing

History of Wall Bracing

Uniform Building Code – 1970

- All exterior walls and main cross stud partitions shall be effectively and thoroughly braced <u>at each end</u>, or as near thereto as possible, and at least <u>every 25 feet of length</u> by on of the following methods:
 - A. Nominal 1-inch by 4-inch...
 - B. Wood boards of 5/8-inch...
 - C. Plywood sheathing...
 - D. Fiberboard sheathing...
 - E. Gypsum sheathing...
 - F. Particleboard sheathing...



History of Wall Bracing

Uniform Building Code - 1994

32-inch alternate braced wall panel added

International Residential Code – 2000

- Bracing percentage requirement added
- Continuous wood structural panel bracing method added

International Residential Code – 2006

- Alternate braced wall pane! adjacent to door or window opening added
- Continuous sheathing 4:1 and 6:1 aspect ratio panels at garage door added



History of Wall Bracing

International Residential Code – 2009

- Methods renamed from number designation to abbreviation
- Wall bracing length determined by the greater length requirement from separate wind and seismic bracing length tables
- Intermittent portal frame at garage added
- Continuous sheathing with structural fiberboard added
- Table of effective braced length for braced panels less than 48 in. long added
- Braced panel end distance imit of 12.5 ft cumulative for SDC A-C with intermittent bracing
- Additional bracing requirements for structures with masonry veneer moved to wall bracing section
- Anchorage for masonry foundations with short wall lengths added
- Angled wall lines added
- Imaginary braced wall lines added



History of Wall Bracing



Bracing Topics





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- 1. What is the maximum offset in a BWL?
 - 4' per side, 8' total
- 2. What is the maximum spacing between the centers of BWP's within a BWL?

25'



3. What is the minimum length of a Method WSP BWP?

4'

4. What is the minimum length of a continuous BWP?

24" (or 16")

5. What is the max. spacing between centers of BWL's?
Wind Design = 60'
Seismic Design = 25' (or 35')





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 Can a bracing panel be less than 48" long?

Yes. Most intermittent - 36"

CS-WSP - 16"

- 7. Is more bracing needed for steeper roofs?
 - Yes
 - > 10' < 15' ridge = + 15%
 - > 15' < 20' ridge = + 30%



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Pacino

8. Are the BWP placement requirements the same for interior and exterior BWL's?

SDC A-C = Yes

- Int. = 12.5' end dist.
- Ext. = 12.5' end dist.


Bracing Topics

Introduction	Getting Started	Bracing Basics	Connections	Other Topics
	<u>Terminology</u> Loads & Limits			
	Irregular Buildings			
	Wind Exposure			
	Connecting the systems			



















Both wind speed and seismic risk must be considered when defining required wall bracing. The required bracing length is the greater of the two bracing lengths.



When considering whether wind or seismic requirements control, a number of factors must be considered.

- Wall bracing length either wind or seismic requirements may
- control. Use the longest required length.
- Hold-downs, Roof Ties, Limits if wind or seismic requirements require additional connections or limits, they must be applied regardless of which requirement set controls.

Wind Requirements

- Wall bracing length
 Braced wall line spacing
 Wall height
 Eave to ridge height
- •Roof ties

Seismic Requirements

- •Wall bracing length
- •Braced wall line spacing
- •Hold-downs
- Material weight limits



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Wind

BWL Spacing = 60' max.

Seismic

SDC C (only applies to townhouses) BWL Spacing = 35' max. Permitted to be = 50' max.

SDC D_0 , D_1 , & D_2 (all dwellings) BWL Spacing = 25' max. Permitted to be = 35' max.

- to accommodate one room not exceeding 900 ft²
- L/W < 3:1
- Increase bracing by factor of 1.4

Tables R602.10.1.2(1),(2),(3), Table R602.10.1.5, & R602.10.1.5



RESISTANCE

В

Length

LOAD

Bracing Topics

Introduction	Getting Started	Bracing Basics	Connections Other Topics	
	Terminology Loads & Limits Irregular Buildings Wind Exposure Connecting the Systems	RANC		





Table R602.3(5)Size, Height and Spacing of Wood Stude

		BEARING WALLS					NONBEARING WALLS	
Stud Size (Inches)	Laterally unsupported stud height (feet)	Maximum spacing when supporting roof-ceiling assembly or habitable attic, only (inches)	Maximum spacing when supporting one floor, plus a roof-ceiling assembly or habitable attic (inches)	Maximum spacing when supporting two floors, a roof-ceiling assembly or habitable attic (inches)	Maximum spacing when supporting one floor height (inches)	Laterally unsupported stud height (feet)	Maximum spacing (inches)	
2 x 3						10	16	
2 x4	10	24	16		24	14	24	
3 x 4	10	24	24	16	24	14	24	
2 x 5	10	24	24		24	16	24	
2 x 6	10	24	24	16	24	20	24	

Table R602.3.1

Maximum Allowable Length of Wood Wall Studs Exposed to Wind Speeds of 100 mph or Less in SDC A-D₂

24 Su	16 Ipporting a roof	12 only	8
Su	ipporting a roof	only	
2 v /		,	
2 × 4	2 x 4	2 x 4	2 x 4
2 x 6	2 x 4	2 x 4	2 x 4
2 x 6	2 x 6	2 x 6	2 x 4
2 x 6	2 x 6	2 x 6	2 x 4
N/A	2 x 6	2 x 6	2 x 6
N/A	N/A	2 x 6	2 x 6
N/A	N/A	N/A	2 x 6
	2 x 6 2 x 6 N/A N/A N/A	2 x 6 2 x 6 2 x 6 N/A 2 x 6 N/A N/A N/A N/A N/A	2 x 6 2 x 6 2 x 6 2 x 6 2 x 6 2 x 6 N/A 2 x 6 2 x 6 N/A 2 x 6 2 x 6 N/A N/A 2 x 6 N/A N/A 2 x 6 N/A N/A A

Table R602.3.1, footnote b





R301.2.2 Seismic provisions.

The seismic provisions of this code shall apply to... SDC C, D_0 , D_1 and D_2 ...

Exception:

Detached one- and two-family dwellings located in Seismic Design Category C are exempt from the seismic requirements of this code.



Wind Requirements Only

Seismic Design Category	One- and two- family	Townhouses
A & B		N/A
С	Exempt	Seismic Req. Apply
D ₀	Seismic Req. Apply	Seismic Req. Apply
D ₁	Seismic Req. Apply	Seismic Req. Apply
D ₂	Seismic Req. Apply	Seismic Req. Apply
Wind	and Seismic Requ	irements
		R301.2.2

R202 TOWNHOUSE

- Three or more attached units
- Units extend from foundation to roof
- Open space on at least two sides





R202 TOWNHOUSE

- Three or more attached units
- Units extend from foundation to roof
- Open space on at least two sides



Not open two sides (therefore, not a townhouse)

R301.2.2



R301.2.2.2.1 Weight of Materials

Average dead loads shall not exceed:

- 15 or 25 psf for roofs/ceiling assemblies
- 10 psf for floor assemblies
- 15 psf for exterior wall assemblies

Wind Requirements

Weight of materials provisions do <u>not</u> apply

Seismic Requirements

Weight of materials provisions apply

Force = Mass x Acceleration



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Irregular building definitions





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R301.2.2.5 Irregular buildings

 "Prescriptive construction ... <u>shall not be used for irregular</u> <u>structures</u> located in Seismic Design Categories C, D₀, D₁, and D₂. Irregular portions of structures shall be designed ... with accepted engineering practice...; design of the remainder of the building shall be permitted to use the provisions of this code."

Wind Requirements

Irregular building provisions do <u>not</u> apply

Seismic Requirements

4

Irregular building provisions apply



Snow Load, R301.2.3 **Design Method** Load <u>< 70 psf</u> **Prescriptive** Engineered > 70 psf .2.3

TABLE R301.2(1) CLIMATIC AND GEOGRAPIC DESIGN CRITERIA

Ground Snow Load	Wind Speed (mph)	Seismic Design Category	Subject to Damage From		Winter Design Temp	Ice Barrier Underlayment Required	Flood Hazards	Air Freezing Index	Mean Annual Temp	
≤70	<110 ⁽¹⁾	A-D ₂	Weathering	Frost Line Depth	Termite					

(1) Wind \leq 100 mph in hurricane-prone regions.



Bracing Topics

Introduction	Getting	Bracing	Connections Other
	Started	Basics	Topics
	Terminology Loads & Limits Irregular Buildings Wind Exposure Connecting the Systems		



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Irregular building definitions





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R301.2.2.2.5 Irregular buildings

"Prescriptive construction ... <u>shall not be used for irregular structures</u> located in Seismic Design Categories C, D_0 , D_1 , and D_2 . Irregular portions of structures shall be designed ... with accepted engineering practice...; design of the remainder of the building shall be permitted to use the provisions of this code."

Wind . Requirements

Irregular building provisions do not apply

Seismic Requirements

4

Irregular building provisions apply

Additional building shape and structural requirements apply







Summary

Setback or Cantilever

- 1. 2" x 10" Joists @ 16" Max.
- 2. Back span to cantilever = 2:1
- 3. Doubled joists at BWP ends
- 4. Continuous rim or approved splice
- 5. Uniform load & 8' max header





2





2

Lateral Support:

When a section of floor or roof is not laterally supported by shear walls or braced wall lines on all edges.











R301.2.2.2.5



Heade Per 1	er Requirements Table R502.5(1)
Maximum Opening length	Minimum Header Requirements
4' 2	Qty 1 – 2" x 12" Qty 2 – 2" x 10"
6' 3	Qty 2 – 2" x 12" Qty 3 – 2" x 10"
8'	Qty 3 – 2" x 12" Qty 4 – 2" x 10"





4

Floor or Roof Opening:

When an opening in a floor or roof exceeds the lesser of 12 feet or 50% of the least floor dimension it must be engineered in high seismic regions.










Getting Started: Irregular Buildings

7

Masonry or Concrete:

When stories above-grade...include masonry or concrete construction.

Exception:

Fireplaces, chimneys, and masonry veneer are permitted by this code. Bracing requirements are defined in section R602.12.







Bracing Topics

Introduction	Getting	Bracing	Connections Other
	Started	Basics	Topics
	Terminology Loads & Limits Irregular Buildings <u>Wind</u> Exposure Connecting the Systems		



R301.2.1.1 Design Criteria

Wind speeds greater than:

- 100 mph in hurricane-prone regions⁽¹⁾, or
- 110 mph elsewhere to be designed using one of the following:
 - WFCM
 - ICC 600
 - ASCE-7
 - AISI S230 (steel)

(1) R202 – Areas vulnerable to hurricanes, defined as the U.S. Atlantic Ocean and Gulf of Mexico coasts where the basic wind speed is greater than 90 mph, and Hawaii, Puerto Rico, Guam, Virgin Islands, and America Samoa.



R301.2.1 Wind limitations

Component and cladding loads for wall coverings, windows, etc. per Table R301.2(2) and adjusted per Table R301.2(3) shall be used...





R301.2.1 Wind limitations



Cladding Failure



Getting Started: Loads & Limits

R301.2.4 Exposure Category.

- 1. Exposure A. Large city centers
- 2. Exposure B. Urban, suburban and wooded areas with numerous, closely spaced obstruction. Exposure B is assumed unless the site clearly meets other category types
- 3. Exposure C. Open terrain with scattered obstructions
- 4. Exposure D. Flat areas exposed to wind flowing over large bodies of water

Table R301.2.3 Height and Exposure Adjustments for
Table R301.2(2).



Exposure A:

Large city centers with at least 50 percent of the buildings having a height in excess of 70 feet for a distance of 0.5 mile upwind from the structure being designed.



Exposure B:

Urban and suburban areas, wooded areas or other terrain with many closely spaced obstructions having the size of single family dwellings or larger.



R301.2.1.4.2



Exposure C (1 of 2):

Open with scattered obstructions or undulations generally less that 30 feet in height extending for 1,500 feet in any direction.





R301.2.1.4.3

Exposure C (2 of 2):

Within Exposure B terrain, but located directly adjacent to open areas of Exposure C for a distance of more than 600 ft.





Exposure D:

Flat, unobstructed areas exposed to wind flowing over open water for at least 1 mile. Extends inland 1,500 feet.





Lateral (Sideways) Load Path









R301.2.1 Wind limitations.



Cladding failures have been more common than bracing failures in recent events



Table R301.2.(2) Component and Cladding Loads.

- Provides values for both positive and negative pressures
- Negative values for leeward side always higher than windward side, nature of wind
- Code does not provide information on capability of products to resist wind pressures – especially suction loads.







Bracing Topics

Introduction	Getting	Bracing	Connections Other
	Started	Basics	Topics
	Terminology Loads & Limits Irregular Buildings Wind Exposure <u>Connectino the</u> Systems		











Nail roof sheathing with 8d ring shank nails 4" and 6" on center.

- Tie gable end walls back to the structure.
- Sheath gable ends with structural wall sheathing.
- Use framing anchors to tie wall system to roof system.
- Nail upper story and lower story structural wall sheathing to common structural rim board.
- Nail wall sheathing with 8d common nails 4" and 6" on center.
- Use continuous structural sheathing over all exterior wall surfaces.
- Extend structural wall sheathing to lap over the sill plate.

Space ¹⁄₂" anchor bolts 32" to 48" on center with 3"x3" plate washers









Bracing Topics



R602.10.2.1 Braced Wall Panel Interior Finish Material

Interior Finish (Gypsum) Required



Exceptions:

- 1. Wall panels braced with Methods GB, ABW, PFG and PFH.
- 2. When an *approved interior finish material with* an in-plane shear resistance equivalent to gypsum board is installed.
- 3. For Methods DWB, WSP, SFB, PBS, PCP and HPS, omitting gypsum wall board is permitted when the length of bracing in Tables R602.10.1.2(1) and R602.10.1.2(2) is multiplied by a factor of 1.5.





R602.10.8 Panel joints Blocking required at horizontal edges of BWP's.

Blocking is required at horizontal edges of BWP's

Exceptions:

- 1. Blocking at horizontal joints is not required in wall segments that are not counted as *braced wall panels*.
- 2. Where the bracing length provided is 2x the minimum length required (Tables R602.10.1.2(1) and R602.10.1.2(2)) blocking at horizontal joints shall not be required in *braced wall panels constructed using Methods WSP*, SFB, GB, PBS or HPS
- 3. When Method GB panels are installed horizontally, blocking of horizontal joints is not required.



Bracing Topics



Intermittent Bracing Methods:

- LIB Let-in diagonal brace
- DWB 3/4" Diagonal wood boards
- WSP 3/8" Wood structural panel
- SFB 1/2" Structural fiberboard
- GB 1/2" Interior gypsum wallboard or gypsum sheathing particleboard
- PBS 3/8" Particleboard sheathing
- PCP Portland cement plaster on studs
- HPS 7/16" Hardboard panel siding
- ABW Alternate braced wall
- PFH Intermittent portal frame
- PFG Intermittent portal frame at garage door openings in SDC A-C



Method LIB – Let-in Brace

- Angled 45 to 60 degrees from horizontal
- Extends continuously from bottom plate to top plate
- 1x4 lumber or approved metal strap
- Application limited
 - 1st and 2nd story in SDC A & B
 - 1st story in SDC C
 - Not permitted in SDC D₀-D₂





Method LIB – Let-in Brace







Method LIB – Let-in Brace

Must extend continuously from bottom plate to top plate





Method DWB – Diagonal Wood Boards

- Wood boards 3/4" (1" nominal) thick applied diagonally
- Studs spaced 24" max.

8' to 12'

4' min. 🖊






Method WSP – Wood Structural Panel



Method SFB – Structural Fiberboard Sheathing

- 1/2" or 25/32" thick
- Studs spaced 16" o.c. max.
- Must conform to ASTM C 208



Method GB – Gypsum Board

- 1/2" min. thick for studs spaced 24" o.c. max.
- 8' length for 1-sided
- 4' length for 2-sided







Method PCP – Portland Cement Plaster

- On studs spaced 16" o.c. max.
- Installed in accordance with R703.6









Method PFG – Intermittent Portal Frame at Garage

- For use in SDC A-C only
- Length of the panel is multiplied by 1.5



- Minimum 24" length
- 5 Header 6' min. to 18' max.



Method PFG – Intermittent Portal Frame at Garage

1 or 2-Story					
Height Min. Iength					
8'	24"				
9'	27"				
10'	30"				

Limited to 10' height maximum Including header height



Method PFG – Intermittent Portal Frame at Garage



Method PFG – Intermittent Portal Frame at Garage





Other bracing methods per code report

- Prefabricated units
- Laminated Kraft-paper board
- Fiberboard in various thicknesses



Bracing Per Code Report

Bracing Topics



Main Concepts

- Allows for narrow BWP's without hold-downs
- BWL's must be fully sheathed with wood structural panel or structural fiberboard sheathing (continuously sheathed)
- Continuous sheathing with WSP is described in R602.10.4
- Continuous sheathing with SFB is described in R602.10.5





Sheathing Requirements:

- 1. Sheath full height areas
- 2. Sheath above and below openings
- 3. Adjacent openings determine minimum BWP length



R602.10.4.2 & R602.10.5











Method CS-WSP

Continuous Sheathing with Wood Structural Panel

- Area above and below openings fully sheathed
- Min 3/8" wood structural panel sheathing

Method CS-SFB

Continuous Sheathing with Structural Fiberboard Sheathing

- Area above and below openings fully sheathed
- Min 1/2" structural fiberboard sheathing



Method CS-WSP

Full-height sheathed wall segments having a width equal or greater than Table R602.10.4.2 are counted toward the total bracing length.

Wall minimum length is based on wall height and height of the adjacent clear opening.



Method CS-WSP Braced Panel Minimum Length

Minimum Length of BWP (inches)			Maximum Opening			
8-ft wall	9-ft wall	10-ft wall	Next to the BWP (% of wall height)			
48	54	<u>></u> \$0	100%			
32	36	40	85%			
24	27	30	67%			



Table R602.10.5.2



Method CS-WSP

Braced Panel Length Requirements for Continuously Sheathed Wall Lines (in)

Method	Adjacent	Wall Height (ft)						
	Opening Height (ft)	8	9	10	11	î2		
	64	24	27	30	33	36		
	68	26	27	30	33	38		
	72	27	27	30	33	36		
	76	30	29	30	33	-36		
	80	32	30	30	33	36		
	84	35	32	32	33	36		
	88	38	35 🗸	33	33	36		
CS-WSP	92	43	37	35	35	36		
	96	48	41	38	36	36		
	100		44	40	38	38		
	104	$\overline{}$	49	43	40	39		
	108		54	46	43	41		
	112			50	45	43		
	116			55	48	45		
	120			60	52	48		

	Adjacent Clear Opening Height (ft)	Wall Height (ft)						
Method		8	9	10	11	12		
	124				56	51		
	128				61	54		
	132				66	58		
C2-W2P	136					62		
	140					66		
	144					72		
CS-G	а	24	27	30	33	36		
CS-PF	<u><</u> 120	16	18	20	22	24		

a. Garage opening adjacent to method CS-G panel shall have header. Max opening height includes header height.

Table R602.10.4.2

Method CS-WSP



R602.10.4.2 & Table R602.10.4.2

Method CS-G Wood structural panel adjacent to garage opening

- Full-height sheathed wall segments to either side of garage openings
- Roof covering dead loads of 3 psf or less
- Applied to one wall only
- •4:1 aspect ratio





Method CS-G



• 4:1 Aspect Ratio (24" min.) Table R602.10.4.1



Method CS-PF Continuous portal frame

Walls on either or both sides of openings in garage may have wall segment with a maximum 6:1 height-to-length ratio.





Method CS-PF



6:1 Aspect Ratio (16" min.)

R602.10.4.1.1





APA Portal Frame Test



Method CS-PF

 Table R602.10.4.1.1: Tension Strap Capacity Required for Resisting Wind Pressures

 Perpendicular to 6:1 Aspect Ratio Walls

MINIMUM	MAXIMUM PONY WALL HEIGHT (feet)	MAXIMUM TOTAL WALL HEIGHT (feet)	MAXIMUM OPENiNG WIDTH (reet)	BASIC WIND SPEED (mph)						
WALL STUD FRAMING NOMINAL SIZE AND GRADE				85	Э0	100	85	90	100	
				Exposure B			Exposure C			
				Tension strap capacity required (lbf)						
	0	10	18	1000	1000	1000	1000	1000	1000	
	1	10	9	1000	1000	1000	1000	1000	1275	
2 × 4 No. 2 Grade			16	1000	1000	1750	1800	2325	3500	
			18	1000	1200	2100	2175	2725	DR	
	2	10	9	1000	1000	1025	1075	1550	2500	
			16	1525	2025	3125	3200	3900	DR	
			18	1875	2400	3575	3700	DR	DR	
	2 12	9	1000	1200	2075	2125	2750	4000		
		12	16	2600	3200	DR	DR	DR	DR	
			18	3175	3850	DR	DR	DR	DR	

Method CS-PF

Table R602.10.4.1.1 cont.: Tension Strap Capacity Required for Resisting WindPressures Perpendicular to 6:1 Aspect Ratio Walls

MINIMUM WALL STUD FRAMING NOMINAL	MAXIMUM PONY WALL HEIGHT	MAXIMUM TOTAL WALL HEIGHT (feet)	MAXIMUM OF ENING WIDTH (feet)	BASIC WIND SPEED (mph)						
				85	90	100	85	90	100	
				Exposure B				Exposure C		
AND GRADE	(ieer)			Tension strap capacity required (lbf)						
2 × 4 No. 2 Grade	4	12	9	1775	2350	3500	3550	DR	DR	
			16	4175	DR	DR	DR	DR	DR	
			9	1000	1000	1325	1375	1750	2550	
2 × 6 Stud Grade	2 12	12	16	1650	2050	2925	3000	3550	DR	
			18	2025	2450	3425	3500	4100	DR	
			9	1125	1500	2225	2275	2775	3800	
	4	12	16	2650	3150	DR	DR	DR	DR	
			18	3125	3675	DR	DR	DR	DR	





APA Whole House Bracing Test




Bracing Basics: Continuous Method

Method CS-SFB

Continuous Sheathing with Structural Fiberboard

- Wall minimum length based on wall height and height of adjacent clear opening
- Maximum wall height = 10'
- Length requirements for braced wail panels in Table R602.10.5.2



Bracing Basics: Continuous Method

Continuous Sheathing Corner Requirements





Bracing Basics: Continuous Method

Continuous Sheathing Corner Requirements



Bracing Topics

Braced Panel Construction Intermittent Bracing Methods Continuous Bracing Methods <u>Mixing Bracing</u> <u>Methods</u>	Introduction	Getting Started	Bracing Basics	Connections	Other Topics
BWP Placement BWL Spacing Required Bracing Length		ethoine	Braced Panel Construction Intermittent Bracing Methods Continuous Bracing Methods Mixing Bracing Methods BWP Placement BWL Spacing Required Bracing Length		

Bracing Basics: Mixing Bracing Methods

R602.10.1.1.2 Braced Wall Panels

BWP method variation permitted from BWL to BWL within a story for intermittent and continuous sheathing

For continuous sheathing, this variation may only be used in SDC A-C with winds \leq 100 mph



Bracing Basics: Mixing Bracing Methods

R602.10.1.1.3 Braced Wall Panels

BWP method variation within a BWL permitted ONLY in SDC A-B and for detached houses in SDC C with intermittent bracing

Not applicable for use with continuous sheathing or dwellings in SDC D₀-D₂



Bracing Topics

Introduction	Getting Started	Bracing Basics	Connections	Other Topics
	Engine	Braced Panel Construction		
		Length		AP

Overview

Wall	Interm	ermittent CS-WS		VSP	CS-SFB	
Bracing Parameter	Wind & SDC A-C	SDC D ₀ -D ₂	Wind & SDC A-C	SDC D ₀ -D ₂	Wind & SDC A-C	SDC D ₀ -D ₂
Panel end distance	12.5' Combined	0' or 8' ^(a)	12.5'	8'	12.5'	Not Permitted
Corner return length	Not rec	luired	24" M	in. ^(b)	32" N	/lin. ^(b)

- (a) 8' with 24" panel at corner or 1,800 lb hold down per R602.10.1.4.1, exception items 1 & 2.
- (b) In lieu of a corner return, an 800 lb hold-down may be fastened to the side of the BWP closest to the corner per R602.10.5.3, exception item 2.

R602.10.1.4 Braced Wall Panel Location (Intermittent Bracing Methods)

Placement Requirements

- BWP to begin no more than "X" feet from the end of a BWL.
- Total combined distance of the end panels to each wall end shall not be more than "X" feet.
- BWP located not more than 25' o.c.
- BWP minimum length in accordance with its method.

Wind	S(C	Seismic
"X" = 12.5'	"X" = 0'	Method WSP only:
Combined distance for		1. 8' w/1,800# hold-down
both ends of wall line		2. 24" panel at corner
		Permitted at both ends

R602.10.1.4

R602.10.4.4 Braced Wall Panel Location Continuous Sheathing with Wood Structural Panel Methods CS-WSP, CS-G, CS-PF

Placement Requirements

- BWPs to begin at each end of a BWL.
- BWP located not more than 25° o.c.
- A minimum 24" corner return at each end of the braced wall line.
 - In lieu of a corner return, an 800 lb hold-down may be fastened to the corner stud.





Does this meet code?

No, width requirement not met.











Offset Limitations

- BWP that are counted as part of a BWL must be in line.
- Offsets out-of-plane up to 4' shall be permitted such that the total out-to-out offset is not more than 8 feet.









How many BWL's?







Bracing Topics

Introduction	Getting Started	Bracing Basics	Connections	Other Topics
	jne	Braced Panel Construction		
		BWL Spacing		
		Required Bracing Length		APA



Bracing Length Tables

2009 – Two bracing length tables

- Wind Table R602.10.1.2(1)
- Seismic Table R602.10.1.2(2)

Required bracing length is the maximum of the two tables' bracing length x all adjustment factors

R602.10.1.2, Table R602.10.1.2(1), Table R602.10.1.2(2) & Table R602.10.1.2(3)



Bracing Requirements Based on Wind Speed

Wind Bracing Table based on these assumptions:

- Wind exposure category B
- Mean roof height of 30 ft
- Eave to ridge height of 10 ft
- Wall height of 10 ft
- Two braced wall lines

Required bracing length is determined by:

- Wind speed
- Story location
- Wall line spacing
- Bracing method

Bracing Requirements Based on Wind Speed – Adjustment Factors

Wind bracing adjustment factors found in the footnotes of Table R602.10.1.2(1)

Footnote:

- a) Wind exposure category
- b) Mean roof height
- c) Eave-to-ridge height
- d) Wall height
- e) Number of braced wall lines
- f) Application of gypsum board finish
- g) Single sided Method GB factor
- h) Method LIB gypsum finish board requirement
- i) Reduction factor for tie downs added to each braced wall panel



Table R602.10.1.2(1)

Adjustment Factor – Wind Exposure Category, Mean Roof Height

Table R602.10.1.2(1), footnote a, b

Number of	Exposure/Height Factor			
Stories	Exposure B	Exposure C	Exposure D	
1	1.0	1.2	1.5	
2	1.0	1.3	1.6	
3	1.0	1.4	1.7	



Adjustment Factor –

Roof Eave-to-Ridge Height

Support	Roof Eave-to-Ridge Height			
Condition	<u><</u> 5'	10'	15'	20'
Roof only	0.7	1.0	1.3	1.6
Roof + floor	0.85	1.0	1.15	1.3
Roof + 2 floors	0.9	1.0	1.1	NP

NP – Not Permitted



Table R602.10.1.2(1), footnote c

Adjustment Factor – Wall Height

Table R602.10.1.2(1), footnote d

Wall Height (ft)	Adjustment Factor
10'	1.0
9'	0.95
8'	0.9
12'	1.1



Adjustment Factor – Number of Braced Wali Lines

Braced wall line

x – Braced wall line spacing

Table R602.10.1.2(1), footnote e

Number of Braced Wall Lines	Adjustment Factor
3	1.30
4	1.45
<u>></u> 5	1.60







Adjustment Factor – No gypsum finish board applied to interior of wall line

Table R602.10.1.2(1), footnote f

Bracing Method	Adjustment Factor
Method LIB	1.8
Methods DWB, WSP, SFB, PBS, PCP, and HPS	1.4



Adjustment Factors – Footnotes g, h, and i

Table R602.10.1.2(1) Footnote	Methods	Requirements	Adjustment Factor
g	GB	Single sided gypsum	2.0
g	GB	Double sided gypsum when fastened 4" o.c. at panel edges and blocked at horizontal joints	0.7
h	LIB	Gypsum board must be attached to at least one side using Section R602.10.2 Method GB fastening requirements	1.0
i	DWB, WSP, SFB, PBS, PCP, and HPS	In one story buildings and top of two or three story buildings, when 800 lb minimum uplift hold-downs are fastened to end studs of each braced wall panel in the braced wall line and to the framing or foundation below.	0.8

Building Type:	All Dwellings
SDC Zone:	A & B

Wind Requirements:AllSeismic Requirements:None

Description	Table R602.10.1.2(1) Footnote	Adjustment Factor
Minimum Total Length of B.W.P.s	Table R602.10.1.2(1)	NA
Wind Exposure	а	1.0 - 1.7
Roof Eave to Ridge Height	b, c	0.7 - 1.6
Wall Height	d	0.9 – 1.1
Number of Braced Wall Lines	е	1.3 – 1.6
No gypsum finish	f	1.4 – 1.8
Approved hold-down added	i	0.8

Table R602.10.1.2(1)





Basic Wind Speed (mph)	Story Location	Braced Wall Line Spacing (ft)	Minimum Total Length of Braced Wall Panels Required Along Each Braced Wall Line			
			Method LIB	Method GB (double ରାର୍ବଟେ)	Methods DWB, WSP, SFB, PBS, PCP, HPS	Continuous Sheathing
≤ 90 (mph)		10	3.5	3.5	2	2
		20	7	7	4	3.5
		30	9.5	9.5	5.5	5
		40	12.5	12.5	7.5	6
		50	15.5	15.5	9	7.5
		60	18.5	18.5	10.5	9
		10	7	7	4	3.5
		20	13	13	7.5	6.5
		30	18.5	18.5	10.5	9
		40	24	24	14	12
		50	29.5	29.5	17	14.5
		60	35	35	20	17

Table R602.10.1.2(1)






Method CS-WSP



Basic Wind		Braced	Minimum Total Length of Braced Wall Panels Required Along Each Braced Wall Line			
Speed (mph)	Story Location	Wall Line Spacing (ft)	Method LIE	Method GB (double ରାର୍ବଟେ)	Methods DWB, WSP, SFB, PBS, PCP, HPS	Continuous Sheathing
		10	3.5	3.5	2	2
		20	7	7	4	3.5
		30	9.5	9.5	5.5	5
		40	12.5	12.5	7.5	6
		50	15.5	15.5	9	7.5
(00) (match)		60	18.5	18.5	10.5	9
<u><</u> 90 (mpn)		10	7	7	4	3.5
		20	13	13	7.5	6.5
		30	18.5	18.5	10.5	9
		40	24	24	14	
		50	29.5	29.5	17	14.5
		60	35	35	20	17

Method CS-WSP

CS-WSP	90 mph	SDC A
Bottom Story	12'	NA

Total Bracing Length = 11.5' vs. 12' Required Bracing is insufficient (From Table – 40' BWL)



Method CS-WSP

CS-WSP	90 mph	SDC A
Bottom Story	11.4'	NA

Total Bracing Length = 11.5' vs. 12.4' Required Bracing is OK 0.95 x 12' = 11.4'

Footnote d. = 9' walls – 95% multiplier; 12.0' x 95% = 11.4'



Method CS-WSP



Basic Wind		Braced	Minimum Total Length of Braced Wall Panels Required Along Each Braced Wall Line			
Speed (mph)	Story Location	Wall Line Spacing (ft)	Method L≀€	Method GB (double sided)	Methods DWB, WSP, SFB, PBS, PCP, HPS	Continuous Sheathing
		10	3.5	3.5	2	2
		20	\sim 7	7	4	3.5
		30	9.5	9.5	5.5	5
		40	12.5	12.5	7.5	6
		50	15.5	15.5	9	7.5
(00) (march)		00	18.5	18.5	10.5	9
<u><</u> 90 (mpn)		10	7	7	4	3.5
		20	13	13	7.5	6.5
		30	18.5	18.5	10.5	9
		40	24	24	14	12
		50	29.5	29.5	17	14.5
	/ `	60	35	35	20	17

Method CS-G





Method CS-PF



Basic Wind	Story Location	Braced Wall Line Spacing (ft)	Minimum Total Length of Braced Wall Panels Required Along Each Braced Wall Line				
Speed (mph)			Method LIB	Method GB (double sided)	Methods DWB, WSP, SFB, PBS, PCP, HPS	Continuous Sheathing	
		10	5.5	5.5	3	3	
		20	10	10	6	5	
		30	14.5 🔿	14.5	8.5	(7)	
		40	18.5	18.5	11	q	
		50	23	23	13	11.5	
<u><</u> 110		60	27.5	27.5	15.5	13.5	
(mph)		10	10.5	10.5	6	5	
		20	19	19	11	9.5	
		30	27.5	27.5	16	13.5	
		40	36	36	20.5	17.5	
		50	44	44	25.5	21.5	
		60	52.5	52.5	30	25.5	

Method CS-PF



Bracing Topics







Wall sheathing in a diagonal wall section may be counted for a wall line's bracing length if the diagonal wall line is 8' or less in length.



Braced Panel Starting Location

Seismic

Up to 8' for Method WSP per illustration below





Collector Design for Bracing in Conventional Construction



Engineered Collector







Collector Design for Bracing in Conventional Construction



Bracing Topics





PE

SWP sole plates and top plates connections per Table R602.3(1)









Figure R602.10.6(1) & Table R602.3(1)





R602.10.6 & Table R602.3(1)







Bracing Topics





Examples

Symbol	Description	Symbol	Description
LIB	Diagonal let-in	ABW	Alternate BWP
DWB	Diagonal wood boards	(PF)	Alt. BWP adj. opening
WSP	Wood structural panel	CS- WSP	Continuous sheathing
SFB	Structural fiberboard	CS-G	Continuous 4:1
GB	Gypsum wallboard	CS-PF	Continuous 6:1
PBS	Particleboard	CS- SFB	Continuous sheathing
PCP	Portland cement		1
HBS	Hardboard		















Description	Adjustment Factors	
Wind Exposure	1.0 - 1.7	1.0 (B)
Roof Eave to Ridge Height	0.7 - 1.6	1.3 (<15')
Wall Height	0.9 – 1.1	.95 (9')
Number of Braced Wall Lines	1.3 – 1.6	1.3 (3)
No gypsum finish	1.4 – 1.8	^{>} 1.0 (No)
Approved hold-down added	0.8	1.0 (No)
< 15' Ridge height	Total multipli	er = 1.61
8' wall		
9' wal!		



Questions?

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